

ADDENDUM NO. TWO (2)
CITY OF BALDWIN, GEORGIA
WATER TREATMENT FACILITY PRETREATMENT SYSTEM
ISSUED May 17, 2024

RE: WATER TREATMENT FACILITY PRETREATMENT SYSTEM
EMI PROJECT No. 20-043

FROM: ENGINEERING MANAGEMENT, INC.
303 SWANSON DRIVE
LAWRENCEVILLE, GA 30043
Greg Bennett, P.E.
770-962-1387

TO: PROSPECTIVE BIDDERS

This Addendum forms a part of the Contract Documents and Drawings and modifies the original bidding documents dated April 2024.

The following items of the Contract Documents are modified as part of this Addendum:

Clarifications:

- **A copy of the geotechnical investigation report for the project is attached.**

Contract Documents and Technical Specifications

- Section 00030 has been modified as follows. See attached revised section 00030 (revisions are in bold and italics)
 - ***The bid opening date has been moved to May 30th at 2:00pm.***
 - ***The project duration has been increased to 450 days.***
- Section 00500 has been modified as follows. See attached revised section 00500 (revisions are in bold and italics)
 - ***The project duration has been increased to 450 days.***
- Section 00510 has been modified as follows. See attached revised page 1 of section 00510 (revisions are in bold and italics)
 - ***The project duration has been increased to 450 days.***

END OF ADDENDUM NO. 2



SAILORS ENGINEERING ASSOCIATES, INC.

1675 SPECTRUM DRIVE • LAWRENCEVILLE, GEORGIA 30043 • TEL (770) 962-5922 • FAX 962-7964

March 13, 2024

Engineering Management, Inc.
Attn: Dennis Williams
303 Swanson Drive
Lawrenceville, GA 30043

Re: Geotechnical Investigation
Pretreatment Building
Baldwin Water Treatment Facility
288 Coldwater Drive
Demorest, Habersham Co., Georgia 30535
SEA Project No. 241-053

Gentlemen:

In accordance with your written authorization, Sailors Engineering Associates, Inc. has completed the geotechnical investigation for the subject project and is pleased to submit this report with our conclusions and recommendations.

GENERAL

The site under investigation is located in southern Habersham County; entirely within the existing, 17.0 acre, Baldwin Water Treatment Plant (addressed as above) in Demorest, Georgia. It is (in plan view) about 0.2 miles northeast of Cannon Bridge Road (State Route 105) and about 1-1/3 miles southwest of downtown Demorest. The site is (mostly) bordered on each side by wooded property or rural, single-family residences (all addressed either along Coldwater Drive, Christy Way Road or Cannon Bridge Road), each surrounded by woods. The city of Demorest has a small piece of land containing a water tank inscribed in the 17.0 acre treatment plant site; in its southwestern portion. Additionally, south of and barely adjacent to the 17.0 acre treatment facility property, is a Cannon Bridge Road addressed, auto repair facility.

An approximate 4,350 square foot, proposed pretreatment building is planned about 20 feet southeast of the facility's largest existing 7,300 (+/-) square foot metal building and about 70 feet southwest of the plant's water tank. The new building is to house two, separate unit, approximate 10 foot tall (each) treatment tanks that reportedly will be supported by two, 6" thick, 20'-2" by 14'-2" concrete equipment pads. The pads are to receive tank operating loads of 0.7 kips per square foot. The finished floor elevation for the new building is to match that of the metal building to its northwest and be at 1392.5 feet.

Currently the proposed building's footprint area has a moderate hill/slope falling northeastward from an elevation of about 1398 feet (along the planned building's southwestern side) to about 1389 feet (along the planned building's northeastern side). The upslope continues southwest of the planned building's footprint and, according to a provided grading plan, will require a maximum cut of about 15 vertical feet (+/-) to achieve planned grades for a building surrounding asphaltic concrete drive. Site drainage is toward the northeast.

The proposed new building pad is vegetated with low grasses/weeds and some bare soil areas and currently contains a large, above ground propane tank that is to be relocated to the west of the planned building. The area around the propane tank (also in the building pad footprint), up until recently, contained a patch of pine trees but during our March 12, 2024 site reconnaissance the trees had been removed (leaving only stumps). The propane tank has an underground utility line that extends from the center of the tank to the southern corner of the plant's largest metal building. In addition, an existing 6 foot chain link fence extends along the southeastern side of the planned building, slightly encroaching into the planned building pad. Our boring B-2 was moved from the planned building corner to a location just inside the fence. Lastly, a shallow drainage swale was observed extending downward toward the north and east that encroaches into the northernmost portion of the planned building. Other than the underground propane gas line, no other utilities were observed in the building pad area during our site reconnaissance.

The purpose of our investigation was to determine the presence of unsuitable soil conditions, near surface ground water or rock that would adversely affect construction costs, and to provide recommendations for site preparation and foundation design.

AREA GEOLOGY

Habersham County, Georgia is located in the physiographic province known as the Blue Ridge/Piedmont. The Piedmont extends from the Hudson River at the north to Alabama at the south. The Piedmont is the least mountainous part of the Appalachian Highlands. The surface of the Piedmont can be described as a broadly undulating or rolling topography with low knobs or ridges, and valleys 30 to 300 feet thick. The underlying crystalline rocks of the Piedmont are metamorphic schists, gneisses, quartzites and slates, and igneous granites and gabbros. According to the Geologic Map of Georgia (1976) the base rock, in the area surrounding the subject site, consists of Metagraywacke/Mica Schist from the Pre-Cambrian-Paleozoic period.

FIELD INVESTIGATION

The field investigation consisted of four soil test borings performed at the locations shown on the attached Boring Plan. The test borings were extended through soils by mechanical drilling procedures using continuous spiral hollow auger flights with a steel fingered Hawthorne bit as the cutting device. The consistencies of the underlying soils were determined by Standard Penetration Testing in accordance with ASTM Specification D1586. Samples were obtained with a standard 1.4 inch I.D., 2.0 inch O.D., split tube sampler as illustrated in the Appendix. The sampler was first seated 6.0 inches to penetrate any loose cuttings; then it was driven an

additional foot with blows of a 140 pound hammer falling 30.0 inches. The number of hammer blows required to drive the sampler each 6.0 inch increment is recorded in the Boring Logs. The number of blows required to drive the sampler the final foot is the standard penetration resistance, an indicator of soil strength.

Water level observations were made during the drilling operations. The elevation of the water table fluctuates during the year and is directly related to the amount of rainfall in the months prior to observation.

The elevations shown on the Logs of Boring were interpolated from a two foot contour interval topographic map and are presumed accurate within +/- 1.0 feet. This map was provided by Engineering Management, Inc. via email on March 7, 2024.

SUBSURFACE CONDITIONS

The subsurface conditions encountered on site, as determined by our drilling program, were as follows:

Topsoil and surface cover: A layer of topsoil approximately 2.0 inches in thickness and consisting of brown sand with a little silt and organics was encountered at each of the boring locations.

Fill materials: Encountered from beneath the topsoil and extending to depths of 1.5 feet to 2.0 feet at soil test borings B-2, B-3 and B-4 were brown sands with varying amounts of silt. These were fill materials that appear to have been placed with a light to moderate compactive effort. Standard penetration resistance values in the fill materials encountered were generally indicative of medium consistency.

Residuum/Saprolite: Beneath the materials mentioned above and extending to planned penetration depth of all borings were light brown, brown, reddish-brown and greyish brown sands with a little silt or silt with a little sand. These were residual materials and saprolites weathered in place from the parent rock. Standard penetration and drilling resistances in these materials were indicative of medium to firm consistency.

Ground water was not encountered in any of the soil test boring locations, but at about 28.5 feet below the boring surface of B-1 (at approximate elevation 1369.5 feet) the soil sampled was very moist and we interpreted this to be the ground water table based on the soil moisture. No mass rock, inferred by auger refusal, or hard drilling materials were encountered in any of the soil test borings to the depths drilled for this investigation. Some firm soils and the boring collapse depths for borings B-2, B-3 and B-4 were encountered as noted on the individual Logs of Boring and in the Conclusions and Recommendations section of this report. Collapse depths can sometimes indicate the nearby past or current presence of a water table. However, in the cases of B-2, B-3 and B-4, we do not think the collapse depths indicate nearby ground water.

CONCLUSIONS AND RECOMMENDATIONS

1. All areas to receive pavement, the structure or fill material should be stripped of organic material and topsoil prior to the commencement of construction. The topsoil should be stockpiled on-site for future use in landscaped areas (if approved by the owner), disposed of in a designated area on-site, or wasted off-site. Topsoil should not be used as structural fill. Until recently, the proposed building pad contained a clump of pine trees around a currently existing (to be relocated) propane tank. The remaining tree stumps and any remnant root mat should be dug up and combined with the organic materials and topsoil/strippings and disposed of similar to the above.
2. Once the site has been stripped, all areas to remain at grade or to receive fill should be proof-rolled with a loaded tandem-axle dump truck in the presence of a representative of the Geotechnical Engineer. Cut areas should be proof-rolled once rough subgrade has been reached. Any soft soils encountered during proof-rolling should be stabilized by compaction or undercut and replaced with suitable compacted materials.
3. We note that our soil test borings B-2, B-3 and B-4 each contained a surficial, small thickness of fill soils. The fill as sampled was not found to contain deleterious matter or be overly soft, but because fill soils are man placed or disturbed soils, their presence can be indicative of soft fill elsewhere. Additionally, the building pad contains a shallow drainage swale that sometimes can contain soft/wet soils. Provided the sampled fill is representative of any remaining fill soils on the site, they should be generally satisfactory for support of the planned development without the need for significant undercutting and replacement with firmer materials; but, we recommend the construction budget contain contingency pricing for a nominal amount of undercutting and firmer material (or additional compacted crushed stone) replacement, should the need arise.
4. All fill material to be utilized on the project should be free of organic or otherwise deleterious materials and compacted to minimum dry densities corresponding to 98% of maximum dry density, and at moisture contents within +/- 3% of optimum moisture content, as obtained by Standard Proctor, ASTM D698.

Fill should be placed in lifts not to exceed 6.0 inches in compacted fill thickness in mass fill areas, and as needed to obtain the required compaction in ditch lines and foundation wall backfill.

All soils encountered on site, with the exception of the topsoil, will be suitable for use as structural fill if they are at +/- 3% of their optimum moisture content and free of organic or otherwise deleterious materials.

5. Cut or fill slopes should not be steeper than 2.0H:1.0V. Fill slopes should be compacted in horizontal lifts not to exceed 6.0 inches in compacted thickness as fill is placed.
6. All fill operations should be monitored by a representative of the Geotechnical Engineer. He should perform sufficient density tests to verify that specified compaction is obtained.

7. Once the above site preparation items have been addressed, the site should be suitable to receive the proposed building founded on spread and/or strip footings designed for a maximum net allowable soil bearing pressure of 2500 pounds per square foot.

Footings widths should be in accordance with the structural engineer's design but should be no less than 18.0 inches for continuous footings and 24.0 inches for individual footings.

The prevailing building code for the subject site requires a minimum embedment depth of 12.0 inches for perimeter foundations. The frost penetration depth for this area is less than 6.0 inches.

Based on the standard penetration resistance values obtained in our limited depth, soil test borings, we have no reason to expect that the seismic site class for the site would be lower than a "D". Site class "D" is based on the average of the standard penetration resistance values for the top 100 feet of the site being between 15 and 50 blows per foot.

We note that the new building is to house two, separate unit, approximate 10 foot tall (each) treatment tanks that reportedly will be supported by two, 6" thick, 20'-2" by 14'-2" concrete equipment pads and that the pads are to receive tank operating loads of 0.7 kips per square foot. The project's structural engineer should design these equipment pads and the supporting building's underlying slab on grade with reinforcing steel to satisfactorily resist these loads/stresses.

8. The base of all footings should be inspected by a representative of the Geotechnical Engineer immediately prior to the placement of reinforcing steel or concrete. He should verify that soil capable of supporting the design bearing pressure has been obtained in each case.
9. All areas to receive pavement should be proof-rolled in the presence of a representative of the Geotechnical Engineer immediately prior to the placement of base course. Soft areas encountered during proof-rolling should be stabilized by compaction or undercut and replaced with suitable compacted fill.
10. Based on wet cuttings at the base of soil test boring B-1, the ground water table at the site is estimated at approximate elevation of 1369.5 feet. At this depth ground water would not be expected to have a significant impact to site development.
11. While some firm soils (residual soils with penetration resistances well above 20 blows per foot) were penetrated in our soil test borings and, in fact, all standard penetration resistances in the soils sampled and expected to support the subject building's footings are in double digits, no mass rock, inferred by auger refusal, or hard drilling materials/weathered rock were encountered in the test borings performed on the site. Neither rock removal nor difficult excavation is anticipated for the development of the site.

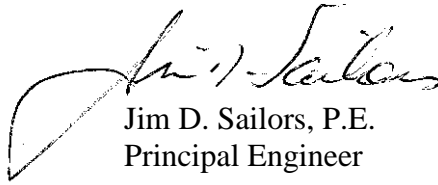
If we can be of further service to you on this project, please contact us at your convenience.

Respectfully submitted,

SAILORS ENGINEERING ASSOCIATES, INC.



Peyton T. Duncan, P.E.
Senior Engineer



Jim D. Sailors, P.E.
Principal Engineer

APPENDIX

SOIL CONSISTENCY DESIGNATIONS

(Based on results of Standard Penetration Tests performed according to ASTM Specification D-1586-84)

NUMBER OF BLOWS (“N”): Shall be defined as the number of blows of a 140 pound hammer falling free a distance of 30 inches required to drive a standard split spoon sampler (2” O.D. and 1.4” I.D.) 1 foot.

When the sample is primarily cohesionless, use the following consistency table:

<u>NUMBER OF BLOWS (N)</u>	<u>CONSISTENCY DESIGNATION</u>
0 - 4	Very Loose
5 - 10	Loose
11 - 30	Medium
31 - 50	Dense
51 or more	Very Dense

When the sample is primarily cohesive, use the following consistency table:

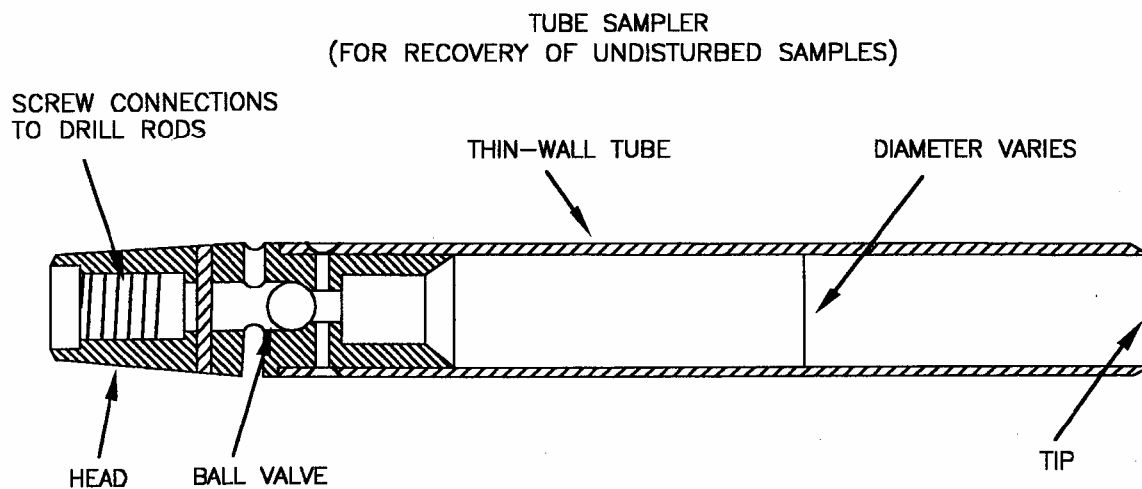
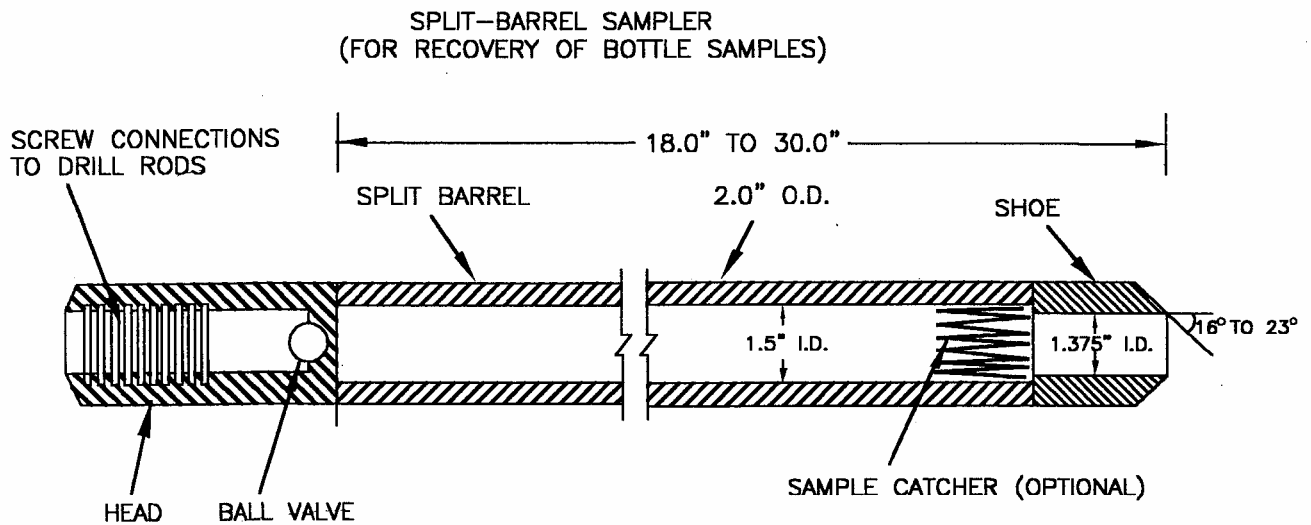
<u>NUMBER OF BLOWS (N)</u>	<u>CONSISTENCY DESIGNATION</u>
0 - 2	Very Soft
3 - 4	Soft
5 - 8	Medium
9 - 15	Stiff
16 - 30	Very Stiff
30 or more	Hard

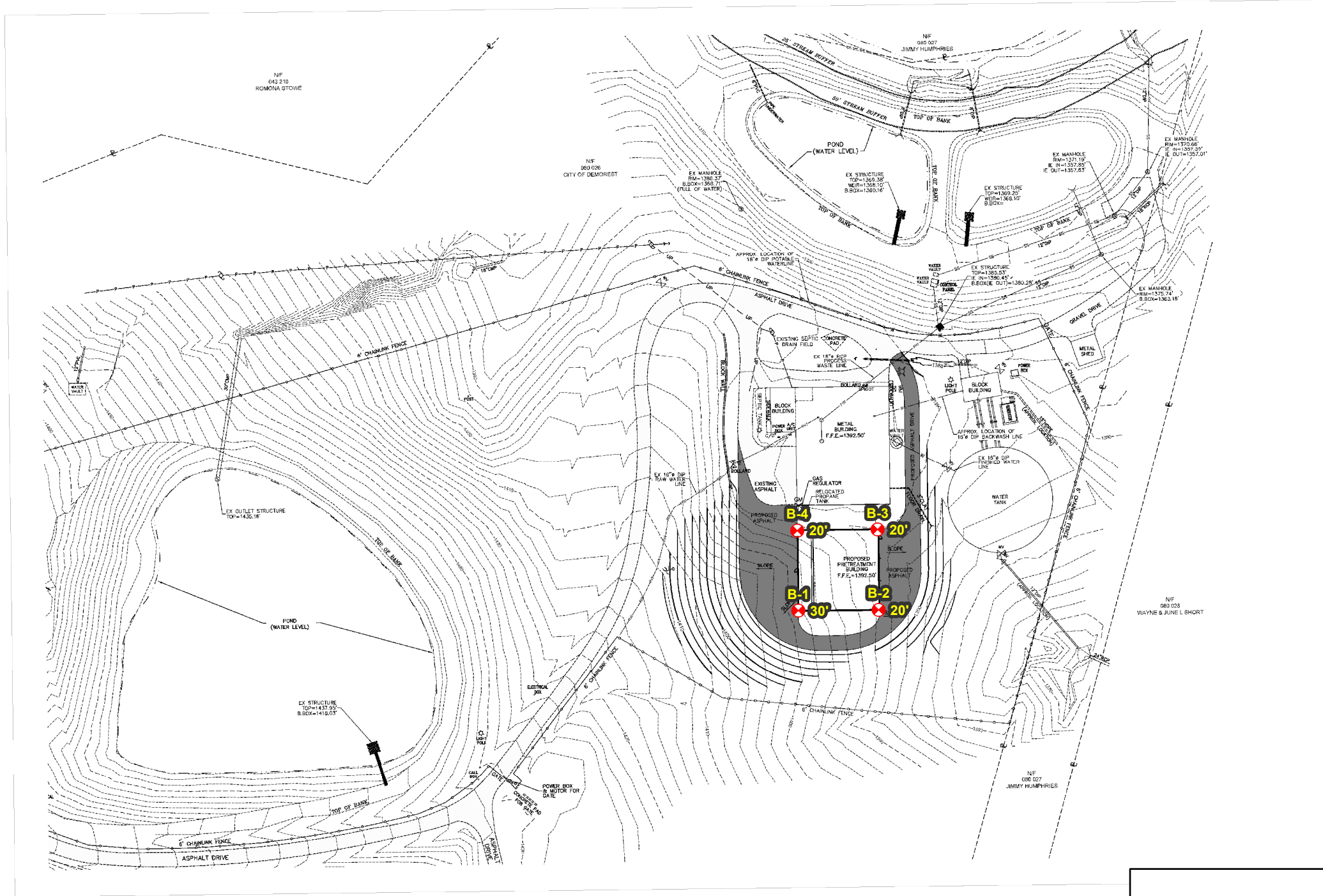
SAMPLING PROCEDURES

Soil Sampling & Penetration Testing is performed in accordance with ASTM D1586-84.

The Standard Penetration Resistance is the number of blows of a 140 pound hammer falling 30 inches to drive a 2.0 inch O.D., 1.375 inch I.D. split barrel sampler one foot.

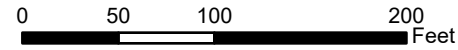
The Undisturbed Sampling Procedure is performed in accordance with ASTM Specification D1587-83.





Legend

⊕ Boring Locations



1" : 100'

Imagery Sources:

SAILORS ENGINEERING ASSOCIATES, INC.
 ENVIRONMENTAL/GEOTECHNICAL
 1675 SPECTRUM DRIVE
 LAWRENCEVILLE, GEORGIA 30043
 (770) 962-5922 FAX: 962-7964



BORING PLAN

BALDWIN WTP PRE-TREATMENT

288 Coldwater Dr
Demorest, Baldwin County, GA

L O G O F B O R I N G

SHEET 1 OF 1
BORING NO.
B-1

CONTRACTED WITH: Engineering Management, Inc.

PROJECT NAME: Baldwin Water Treatment Facility

JOB NO. DATE:
241-053 03/13/24

LOCATION: 288 Coldwater Drive - Demorest, Georgia

ELEV. (ft)	DESCRIPTION	DEPTH IN FEET	SAMPLES			Diedrich MudBug with AutoHammer
			No.	TYPE	BLOWS/6"	NOTES
1398.0	Topsoil: 2.0" brown sand with a little silt and organics					
	Reddish brown silt with some sand (residuum)	1	1	SS	2-4-5	Drilling medium
		2				
	Light brown sand with a little silt (saprolite)	3	2	SS	3-6-10	
		4				
		5				
		6	3	SS	5-12-13	
		7				
		8				
	Light grayish brown sand with a trace of silt (saprolite)	9	4	SS	5-7-11	
		10				
		11				
		12				
		13				
		14	5	SS	5-8-11	
		15				
		16				
		17				
		18				
		19	6	SS	4-6-10	
		20				
		21				
		22				
		23				
		24	7	SS	5-7-9	
		25				
		26				
		27				
		28	▽			
1369.5		29	⚡			
		30	8	SS	5-9-9	Sample very moist; Ground water estimated at 28.5 feet based on the soil cuttings.
1368.0	Boring terminated at 30.0 feet	31				
		32				
		33				
		34				
		35				
		36				
		37				
		38				
		39				
		40				
		41				

SEA

L O G O F B O R I N G

SHEET 1 OF 1
BORING NO.
B-3

CONTRACTED WITH: Engineering Management, Inc.

JOB NO. DATE:
241-053 03/13/24

PROJECT NAME: Baldwin Water Treatment Facility

LOCATION: 288 Coldwater Drive - Demorest, Georgia

ELEV. (ft)	DESCRIPTION	DEPTH IN FEET	SAMPLES			Diedrich MudBug with AutoHammer
			No.	TYPE	BLOWS/6"	NOTES
1389.0	Topsoil: 2.0" brown sand with a little silt and organics					
	Brown sand with some silt (fill)	1	1	SS	4-5-5	Drilling medium
	Reddish brown silt with a little sand (residuum)	2				
		3	2	SS	5-7-10	
		4				
	Brown sand with a little silt (saprolite)	5				
		6	3	SS	5-7-8	
		7				
		8				
		9				
		10	4	SS	6-9-10	
		11				
		12				
		13				
		14	5	SS	7-9-13	
		15				
		16				
		17				
		18				
		19				
		20	6	SS	9-15-20	Drilling firm Boring collapsed at 12.1 feet. No ground water encountered above that depth at 0 hours after drilling.
1369.0	Boring terminated at 20.0 feet	21				
		22				
		23				
		24				
		25				
		26				
		27				
		28				
		29				
		30				
		31				
		32				
		33				
		34				
		35				
		36				
		37				
		38				
		39				
		40				
		41				

L O G O F B O R I N G

SHEET 1 OF 1
BORING NO.
B-4

CONTRACTED WITH: Engineering Management, Inc.

JOB NO. **241-053** DATE: **03/13/24**

PROJECT NAME: Baldwin Water Treatment Facility

LOCATION: 288 Coldwater Drive - Demorest, Georgia

ELEV. (ft)	DESCRIPTION	DEPTH IN FEET	SAMPLES			Diedrich MudBug with AutoHammer
			No.	TYPE	BLOWS/6"	NOTES
1393.0	Topsoil: 2.0" brown sand with a little silt and organics					
	Brown sand with some silt (fill)	1	1	SS	2-2-5	Drilling medium Drilling firm Boring collapsed at 10.8 feet. No ground water encountered above that depth at 0 hours after drilling.
		2				
	Light grayish brown sand with a little silt (saprolite)	3	2	SS	8-11-14	
		4				
		5				
		6	3	SS	6-11-13	
		7				
		8				
		9				
		10	4	SS	8-12-21	
		11				
		12				
	13					
	14					
	15	5	SS	11-17-25		
	16					
	17					
	18					
	19	6	SS	11-13-18		
1373.0	Boring terminated at 20.0 feet	20				
		21				
		22				
		23				
		24				
		25				
		26				
		27				
		28				
		29				
		30				
		31				
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		41				

ADVERTISEMENT FOR BIDS
Water Treatment Facility Pretreatment System
for the
City of Baldwin, Georgia

Sealed bids will be received by the City of Baldwin, Georgia (OWNER), for furnishing all materials, labor, tools, equipment, and any other miscellaneous items necessary for the construction of Water Treatment Facility in Baldwin, Georgia.

A Pre-Bid Conference will be held at 10:00 a.m. local time on May 14, 2024, at the Baldwin Water Treatment Facility located at 288 Cold Water Drive, Demorest, Georgia 30535. Representatives of OWNER and ENGINEER will be present to discuss the Project. Bidders are encouraged to attend and participate in the Conference. ENGINEER will transmit to all prospective Bidders of record such Addenda as ENGINEER considers necessary in response to questions arising at the Conference. Oral statements may not be relied upon and will not be binding or legally effective.

Bids will be received at City Hall of Baldwin, 186 Highway 441, Baldwin, Georgia 30511 until 2:00 p.m. on **May 30, 2024**. Any bid received after said time and date of bid opening will not be considered by OWNER. Bids will be publicly opened and read aloud at this time and location. All bids will be evaluated by OWNER and the project will be awarded, if it is awarded, within sixty (60) days of the bid opening. If a bidder is not selected within sixty (60) days of the bid opening, any bidder that is determined by the OWNER to be unlikely of being selected for contract award shall be released from their bid.

The award of this bid proposal, if awarded, shall be based on several criteria, including prices bid, experience in the work proposed, and references. Refer to Article 14 of Section 00100 for criteria and award method.

The Project consists of, but is not limited to the following major elements:

Construction of a new Package Pretreatment System at the existing 4.0MGD Water Treatment Facility. Major components include a 2800 GPM packaged water pretreatment system, pre-engineered metal building, chemical feed systems, electrical, site grading and drainage, piping, and other miscellaneous appurtenances.

Time of completion for all work associated with this project shall be **four hundred fifty (450)** consecutive calendar days from the date of a written "Notice to Proceed" from OWNER.

Copies of Contract Documents, Specifications, and Construction Drawings may be obtained at the office of Engineering Management, Inc., 303 Swanson Drive, Lawrenceville, Georgia 30043, upon payment of a non-refundable cost of \$250.00. Checks shall be made payable to Engineering Management, Inc.

For purchasing documents regarding this project, you may contact Joyce Sapp at 770-962-1387 or jsapp@eminc.biz. For technical questions regarding this project, you may contact Greg Bennett at 770-962-1387, or gbennett@eminc.biz.

The Information for Bidders, Bid Proposal Form, Form of Agreement, Drawings, Specifications, and forms of Bid Bond, Performance Bond, Payment Bond, Bidders Qualification Forms, and other Contract Documents may be examined at the following locations:

OWNER:

City of Baldwin
186 Highway 441 Bypass
Baldwin, GA 30511
(706) 778-6341 (T)
(706) 776-7970 (F)

ENGINEER:

Engineering Management, Inc.
303 Swanson Drive
Lawrenceville, GA 30043
770-962-1387 (T)
770-962-8010 (F)

A list of persons who purchased Contract Documents from Engineering Management, Inc. will be available from the Engineer ONLY through their website at www.eminc.biz, email, facsimile transmission or U.S. Mail.

OWNER requires a bid bond* or a certified cashier's check in the amount of five percent (5%) of the total bid to be enclosed with the bid at the time of bid opening. Cashier's check will be made payable to the City of Baldwin.

The successful bidder will be required to furnish OWNER with Insurance, Workman's Compensation Insurance, and Performance and Payment Bonds* in the amount of one-hundred percent (100%) of the total bid.

Each bid must be submitted in a SEALED ENVELOPE, addressed to the OWNER. Each sealed envelope containing a Bid must be plainly marked on the outside as, "Water Treatment Facility Pretreatment System, City of Baldwin, Georgia". If bid is forwarded by mail, the sealed envelope containing the Bid must be enclosed in a separate mailing envelope to the attention of the OWNER at the address previously given.

All Bids must be made out on the bid form of the type bound in the Contract Documents, in accordance with the instructions in the Information for Bidders. No interlineation, additions, or deletions shall be made in the proposal form by the BIDDER.

Any and all Bids received without the aforementioned qualification criteria enclosed, will be returned to the BIDDER.

CONTRACTORS and SUBCONTRACTORS bidding on this Project will be required to comply with all Federal, State, and local laws.

OWNER reserves the right to waive any informalities or to reject any or all Bids, to evaluate Bids, and to accept any Bid which in its opinion may be in the best interest of the OWNER. No Bid will be rejected without just cause.

Successful Bidder will be required to perform WORK as the Prime Contractor. WORK performed by Prime Contractor shall be at a minimum 30% of the contract value.

No BIDDER may withdraw his bid within the time limit specified in the Instruction to Bidders (Section 00100).

The OWNER has acquired all necessary State and Federal permits in connection with this project.

*** Surety and insurance companies must have an AM Best rating of A-6 or greater, be listed in the Federal Registry of Companies holding Certificates of Authority and Acceptable Sureties on Federal Bonds, be licensed by the Georgia Insurance Department and the Georgia Secretary of State to do business in the State of Georgia. Surety companies executing Bonds must appear on the Treasury Department's most current list (Circular 570 as amended) For Performance Bonds less than \$300,000 an irrevocable Letter of Credit from a bank as defined in O.C.G.A. Code Section 7-1-4 may be submitted in lieu of a bond. In lieu of a Payment Bond, a Cashier's Check, Certified Check, or Cash may be submitted in an amount not less than the total amount payable by the terms of the CONTRACT.**

Stephanie Almagno, Mayor
City of Baldwin, Georgia

END OF SECTION

NOTICE OF AWARD

TO: _____

Project Description

The site of the proposed work is in Baldwin, Georgia. The project consists of, but is not limited to, the following major elements:

Construction of a new Package Pretreatment System at the existing 4.0MGD Water Treatment Facility. Major components include a 2800 GPM packaged water pretreatment system, pre-engineered metal building, chemical feed systems, electrical, site grading and drainage, piping, and other miscellaneous appurtenances.

CONTRACTOR agrees to commence work on or before a date to be specified in a written "Notice to Proceed" of the OWNER and to fully complete all work associated with this project in a total construction time of *four hundred fifty (450)* consecutive calendar days from the date of the "Notice to Proceed" from the OWNER.

The OWNER has considered the Bid submitted by you for the above-described WORK in response to its Advertisement for Bids and has decided to award you the Contract.

You are hereby notified that your Bid has been accepted for the Contract Price of \$_____ for Water Treatment Facility Pretreatment System which is based on the Units/Lump Sum Prices shown on the Bid Form (Section 00300).

You are required by the Information for Bidders to execute the Agreement and furnish the required CONTRACTOR's Performance and Payment Bonds and Certificates of Insurance within ten (10) calendar days from the date of this Notice.

If you fail to execute said Agreement and to furnish said Bonds within ten (10) calendar days from the date of this Notice, said OWNER will be entitled to consider all your rights arising out of the OWNER's acceptance of your Bid as abandoned and as a forfeiture as may be granted by law.

You are required to return an acknowledged copy of this Notice of Award to the OWNER.

Dated this ___ day of _____, 20__.

CITY OF BALDWIN

BY: _____
Stephanie Almagno, Mayor

ACKNOWLEDGEMENT OF NOTICE

CONTRACTOR

BY: _____

DATE: _____

TITLE: _____

END OF SECTION

AGREEMENT

THIS AGREEMENT is dated as of the _____ day of _____ in the year ____, by and between the City of Baldwin, Georgia (hereinafter called OWNER), and _____ (hereinafter called CONTRACTOR). OWNER and CONTRACTOR, in consideration of the mutual covenants hereinafter set forth, agree as follows:

The Project consists of, but is not limited to the following major elements:

Construction of a new Package Pretreatment System at the existing 4.0MGD Water Treatment Facility. Major components include a 2800 GPM packaged water pretreatment system, pre-engineered metal building, chemical feed systems, electrical, site grading and drainage, piping, and other miscellaneous appurtenances.

ARTICLE 1 - WORK

CONTRACTOR shall complete all WORK as specified or indicated in the CONTRACT DOCUMENTS. The WORK described previously includes all material, labor, tools, equipment, and any other miscellaneous items necessary to complete the work as described in the Technical Specifications and Construction Drawings.

ARTICLE 2 - ENGINEER

The Project has been designed by Engineering Management, Inc., 303 Swanson Drive, Lawrenceville, Georgia 30043, who is hereafter referred to as ENGINEER, and who will assume all duties and responsibilities and will have the rights and authority assigned to ENGINEER in the CONTRACT DOCUMENTS in connection with completion of the WORK in accordance with the CONTRACT DOCUMENTS.

ARTICLE 3 - CONTRACT TIME

- 3.1 CONTRACTOR agrees to commence WORK within ten (10) days of a date to be specified in a written "Notice to Proceed" from the OWNER and to fully complete the contract in a total construction time of **four hundred fifty (450)** consecutive calendar days from the date of the "Notice to Proceed" from the OWNER.
- 3.2 Time for Completion: OWNER and CONTRACTOR recognize that time is the essence of this Agreement and that OWNER will suffer financial loss if the WORK is not substantially complete within the time specified in Paragraph 3.1 above, plus any extension thereof allowed in the General Conditions. They also recognize the delays, expense and difficulties involved in proving in a legal or arbitration proceeding the actual loss suffered by OWNER if the WORK is not substantially complete on time. Accordingly, OWNER and CONTRACTOR understand and recognize that it is impossible to conclusively assess damages to the OWNER for the failure of the CONTRACTOR to substantially complete the project in a timely manner. Therefore, the OWNER and CONTRACTOR have agreed that a reasonable amount of damages for each day that the project remains incomplete after the contract time (as stated in paragraph 3.1) has expired would be the amount of \$1000.00.

ARTICLE 4 - CONTRACT PRICE

OWNER shall pay CONTRACTOR for performance of the WORK in accordance with the CONTRACT DOCUMENTS in current funds as follows: (Amount in words _____) (\$ _____).